ABBREVIATIONS

∮	AND
@	AT
A.B.	ANCHOR BOLT
ABV.	ABOVE
L	ANGLE
A/C	AIR CONDITIONING
ALT.	ALTERNATE
ARCH.	ARCHITECTURAL
B.F.	BRACED FRAME
BLDG.	BUILDING
BLKD.	BLOCKED
BLKG.	BLOCKING
BLW.	BELOW
BM.	BEAM
B.N.	BOUNDARY NAILING
B.O.F.	BOTTOM OF FOOTING
BRG.	BEARING
BTM.	BOTTOM
BTWN.	BETWEEN
C=	CAMBER
C.I.P.	CAST IN PLACE
C.J.	CONSTRUCTION JOINT
℄	CENTER LINE
CLG.	CEILING
CLR.	CLEAR
C.M.U.	CONCRETE MASONRY UNIT
COL.	COLUMN
CONC.	CONCRETE
CONN.	CONNECTION
CONST.	CONSTRUCTION
CONT.	CONTINUOUS
CSK	COUNTERSINK
⌀	DIAMETER
DBL.	DOUBLE
DIA.	DIAMETER
DIAG.	DIAGONAL
DO, do	DITTO
DWG.	DRAWING
DWL.	DOWEL
(E)	EXISTING
EA.	EACH
E.F.	EACH FACE
E.J.	EXPANSION JOINT
EL.	ELEVATION (DATUM)
ELEC.	ELECTRICAL
ELEV.	ELEVATOR
EMBED.	EMBEDMENT
E.N.	EDGE NAILING
E.S.	EACH SIDE
E.W.	EACH WAY
EXIST.	EXISTING
EXT.	EXTERIOR
FIN.	FINISH
FLG.	FLANGE
FLR.	FLOOR
F.N.	FIELD NAILING
FND.	FOUNDATION
F.O.	FACE OF
F.O.C.	FACE OF CONCRETE
F.O.M.	FACE OF MASONRY
F.O.S.	FACE OF STUD
F.P.	FULL PENETRATION
FRMG.	FRAMING
FTG.	FOOTING
F.V.	FIELD VERIFY
GA.	GAGE
GALV.	GALVANIZED
GL	GLUED LAMINATED BEAM
GLB	GLUED LAMINATED BEAM
GRD.	GRADE
(H)	HORIZONTAL
HORIZ.	HORIZONTAL
HDR.	HEADER
HD	HOLDOWN
H.D.G.	HOT DIPPED GALVANIZED
HDR.	HEADER
HGR.	HANGER
HT.	HEIGHT
H.S.B.	HIGH STRENGTH BOLT
INT.	INTERIOR
JT.	JOINT
JST.	JOIST
LB.	POUND
LG.	LONG
LONGIT.	LONGITUDINAL
LT. WT.	LIGHTWEIGHT
MAX.	MAXIMUM
M.B.	MACHINE BOLT
MECH.	MECHANICAL
MIN.	MINIMUM
MTL.	METAL
(N)	NEW
NLG.	NAILING
O.C.	ON CENTER
O.H.	OPPOSITE HAND
OPNG.	OPENING
d	PENNY
PC.	PIECE
P.C.C.	PRECAST CONCRETE
ℙ	PLATE
PLYWD.	PLYWOOD
P.T.	PRESSURE TREATED
RWD.	REDWOOD
REINF.	REINFORCING
REQ'D.	REQUIRED
RF.	ROOF
S.C.	SHEAR CONNECTOR
SCHED.	SCHEDULE
SHTG.	SHEATHING
SIM.	SIMILAR
SMS	SHEET METAL SCREW
S.O.G.	SLAB ON GRADE
SPCG.	SPACING
SQ.	SQUARE
STAGG.	STAGGERED
STGRD.	STAGGERED
STIFF.	STIFFENER
STL.	STEEL
STR.	STRUCTURAL
STRUCT.	STRUCTURAL
T ∮ B	TOP ∮ BOTTOM
TG	TAPERED GIRDER
TSG	TAPERED STEEL GIRDER
THD.	THREAD
T.N.	TOE NAILING
T.O.C.	TOP OF CONCRETE
T.O.F.	TOP OF FOOTING
T.O.P.	TOP OF PARAPET
T.O.S.	TOP OF STEEL
T.O.W.	TOP OF WALL
TRANS.	TRANSVERSE
TS	STRUCTURAL TUBING
TS	(TUBE STEEL)
TYP.	TYPICAL
UNBLKD.	UNBLOCKED
U.O.N.	UNLESS OTHERWISE NOTED
(V)	VERTICAL
VERT.	VERTICAL
W.P.	WORK POINT
W/	WITH
WHS	WELDED HEADED STUD
WT.	WEIGHT
WTS	WELDED THREADED STUD
W.W.F.	WELDED WIRE FABRIC

REINFORCED CONCRETE (REGULAR WEIGHT 145 PCF)

1. CONCRETE STRENGTHS: THE CONCRETE STRENGTH SHOWN IN THE FOLLOWING TABLE IS THE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS AND THE SLUMP IS THE MAXIMUM IN INCHES.

ITEM OF CONSTRUCTION	STRENGTH (PSI)	SLUMP (INCHES)
FOUNDATIONS	2,500	4.5
SLAB ON GRADE	2,500	3.5
NON-SHRINK GROUT	5,000	
STEM WALLS	2,500	4

2. REINFORCING STEEL: ASTM-A615, GRADE 60 #4 AND LARGER, #3 BAR GRADE 40: A706 FOR ALL WELDED REBAR.
3. CONCRETE PROTECTION FOR REINFORCEMENT. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

	MIN. COVER (INCHES)
A.) CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3
B.) CONCRETE EXPOSED TO EARTH OR WEATHER: NO. 6 THROUGH NO. 18 BAR	2
NO. 5 BAR AND SMALLER	1 1/2
C.) CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, WALLS: NO.11 BAR AND SMALLER	3/4
BEAMS, COLUMNS: PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1 1/2

4. ANCHOR BOLTS AND DOWELS: SECURELY HELD IN PLACE PRIOR TO PLACING CONCRETE. BOLTS SHALL BE FULL DIAMETER WITH CUT THREADS.
5. VERTICAL REINFORCEMENT SHALL BE DOWELED TO THE SUPPORTING MEMBERS WITH THE SAME SIZE AND SPACING OF REINFORCEMENT AS CALLED FOR IN THE DRAWINGS OR STANDARD NOTES.
6. SPLICES IN REINFORCING: THE MINIMUM LAP SHALL BE TENSION LAP SPLICE CLASS "B" PER SCHEDULE.
7. SLAB REINFORCING: SPLICE TOP BARS AT MID SPAN. SPLICE BOTTOM BARS AT SUPPORTS.
8. MINIMUM BEARING ON WALLS: FULL THICKNESS OF WALL, OR 12" MINIMUM. FOR BEAMS, 6" FOR SLABS.
9. SPACER TIES: FURNISH #3 TIES AT 24" IN ALL BEAMS AND FOOTINGS, UNLESS OTHERWISE SHOWN IN DETAILS OR PROVIDE WIRE TIES TO STAKES TO SECURE REINF.
10. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS OF PIPES, DUCTS, VENTS AND SIMILAR OPENINGS
11. CHAMFER: 3/4" ON ALL EXPOSED CORNERS.
12. REINFORCING STEEL SHOP DRAWINGS AND PLACEMENT DRAWINGS: REVIEWED BY THE ARCHITECT BEFORE FABRICATION.
13. PROPOSED LOCATIONS OF ANY CONSTRUCTION JOINTS IN SUPPORTED SLABS AND WALLS SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL PRIOR TO CONSTRUCTION.
14. THE ARCHITECT SHALL BE NOTIFIED 24 HOURS IN ADVANCE OF ALL CONCRETE PLACEMENT.
15. NO BACKFILL SHALL BE POURED AGAINST ANY WALL UNTIL THE WALL HAS REACHED FULL STRENGTH ∮ THE CONNECTING SLABS (WHERE OCCUR) HAVE BEEN CAST ∮ HAVE ALSO REACHED FULL STRENGTH.

THE CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTTION OF THE SEISMIC-FORCE-RESISTING SYSTEM SHALL SUBMIT A WRITTEN 'STATEMENT OF RESPONSIBILITY' TO THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM

1. CUTTING ∮ NOTCHING WOOD STUDS (SECTION 2320.11.9)

BEARING WALL STUDS:

ANY WOOD STUD MAY BE CUT OR NOTCHED TO A DEPTH NOT EXCEEDING 25 PERCENT OF ITS WIDTH.

TABLE 1

STUD SIZE	MAXIMUM DEPTH OF EDGE CUT OR NOTCH	MINIMUM WOOD WIDTH REMAINING AFTER CUT OR NOTCH
ALL MEASUREMENTS IN INCHES		
2X4	7/8	2 5/8
2X6	1 3/8	4 1/8
3X4	7/8	2 5/8

2. BORED HOLES IN WOOD STUDS (SECTION 2320.11.10)

BEARING WALL STUDS:

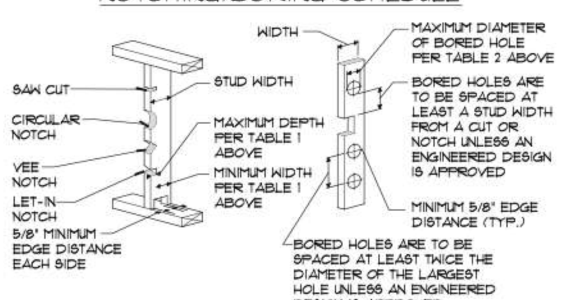
A HOLE NOT GREATER IN DIAMETER THAN 40 PERCENT OF THE STUD WIDTH MAY BE BORED IN ANY WOOD STUD. BORED HOLES SHALL NOT BE LOCATED AT THE SAME SECTION OF THE STUD AS A CUT OR NOTCH.

TABLE 2

STUD SIZE	MAXIMUM DIAMETER OF BORED HOLE	MIN. EDGE DISTANCE ON EACH SIDE OF HOLE AFTER BORING
ALL MEASUREMENTS IN INCHES		
2X4	1 3/8	5/8
2X6	2 1/4	5/8
3X4	1 3/8	5/8

NOTE:
BORED HOLES NOT GREATER THAN 60% OF STUD WIDTH ARE PERMITTED IN STRUCTURAL WALL WHERE EACH BORED HOLE IS DOUBLED, PROVIDED THAT NOT MORE THAN TWO SUCCESSIVE STUDS ARE SO BORED.

NOTCHING/BORING SCHEDULE



STRUCTURAL
STUD WALL NOTCHING/ BORING (2)
SCALE: 3/4" = 1'-0"

TABLE 2304.9.1 FASTENING SCHEDULE (MINIMUM UNLESS NOTED OTHERWISE)

JOIST TO SILL OR GIRDER, TOENAIL....3-8d, 3-3" x Ø.131" nails, 3-3" 14 gage staples
BRIDGING TO JOIST, TOENAIL EACH END....2-8d, 2-3" x Ø.131" nails, 2-3" 14 gage staples
1"x6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL....2-8d
WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST, FACE ∮ NAIL....3-8d
2" SUBFLOOR TO JOIST OR BLOCKING, BLIND ∮ FACE NAIL....2-16d Com
SOLE PLATE TO JOIST OR BLOCKING, TYPICAL FACE NAIL....16d @ 16" O.C., 3" x Ø.131" nails @ 8" o.c., 3" 14 gage staples @ 12" o.c.
SOLE PLATE TO JOIST OR BLOCKING, AT BRACED WALL PANEL....16d @ 16" O.C., 4-3" x Ø.131" nails @ 16" o.c., 4-3" 14 gage staples @ 16" o.c.
TOP PLATE TO STUD, END NAIL....2-16d Com, 3-3" x Ø.131" nails, 3-3" 14 gage staples
STUD TO SOLE PLATE....TOENAIL 4-8d, 4-3" x Ø.131" nails, 3-3" 14 gage staples; END NAIL 2-16d Com, 3-3" x Ø.131" nails, 3-3" 14 gage staples
DOUBLE STUDS, FACE NAIL....16d @ 24" o.c., 3" x Ø.131" nails @ 8" o.c., 3" 14 gage staples @ 8" o.c.
DOUBLED TOP PLATES, TYPICAL FACE NAIL....16d @ 16" o.c., 3" x Ø.131" nails @ 12" o.c., 3" 14 gage staples @ 12" o.c.
DOUBLE TOP PLATES, LAP SPLICE....8-16d Com, 12-3" x Ø.131" nails, 12-3" 14 gage staples
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL....3-8d, 3'-3" x Ø.131" nails, 3-3" 14 gage staples
RIM JOIST TO TOP PLATE, TOENAIL....8d @ 6" o.c.3" x Ø.131" nails @ 6" o.c., 3" 14 gage staples @ 6" o.c.
TOP PLATES, LAPS ∮ INTERSECTIONS, FACE NAIL....2-16d Com, 3-3" x Ø.131" nails, 3-3" 14 gage staples
CONTINUOUS HEADER, TWO PIECES....16d Com @ 16" o.c. along each edge
CEILING JOISTS TO PLATE, TOENAIL....3-8d, 3-3" x Ø.131" nails, 5-3" 14 gage staples
CONTINUOUS HEADER TO STUD, TOENAIL....4-8d
CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL....3-16d Com min. 4-3" x Ø.131" nails 4-3" 14 gage staples; SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1
CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL....3-16d Com min 4-3" x Ø.131" nails 4-3" 14 gage staples; SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1
RAFTER TO PLATE, TOENAIL....3-8d 3-3" x Ø.131" nails 3-3" 14 gage staples; SEE SECTION 2308.10.1, TABLE 2308.10.1
1" BRACE TO EACH STUD ∮ PLATE, FACE NAIL....2-8d 2-3" x Ø.131" nails 3-3" 14 gage staples
1"x8" SHEATHING TO EACH BEARING, FACE NAIL....3-8d
WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL....3-8d
BUILT-UP CORNER STUDS....16d Com @ 24" o.c., 3" x Ø.131" nails, 3" 14 gage staples
BUILT-UP GIRDER AND BEAMS, FACE NAIL (T ∮ B ∮ STGR'D OPP SIDES)....20d @ 32" o.c. 3" x Ø.131" nails @ 24" o.c. 3" 14 gage staples @ 24" o.c.
(ENDS AND AT EACH SPLICE)....2-20d 3-3" x Ø.131" nails 3-3" 14 gage staples

Com = COMMON
RA = RAFTER

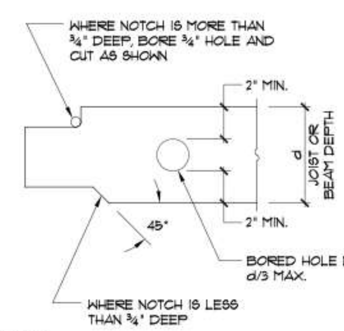
2" PLANKS @ EACH BEARING....16d Com
COLLAR TIE TO RAFTER, FACE NAIL....3-10d, 4-3" x Ø.131" nails, 4-3" 14 gage staples
JACK RAFTER TO HIP, TOE NAIL....3-10d, 4-3" x Ø.131" nails, 4-3" 14 gage staples
JACK RAFTER TO HIP, FACE NAIL....2-16d Com, 3-3" x Ø.131" nails, 3-3" 14 gage staples
ROOF RA. TO 2x RIDGE BEAM, TOE NAIL....2-16d Com, 3-3" x Ø.131" nails, 3-3" 14 gage staples
ROOF RA. TO 2x RIDGE BEAM, FACE NAIL....2-16d Com, 3-3" x Ø.131" nails, 3-3" 14 gage staples
JOIST TO BAND JOIST, FACE NAIL....3-16d Com, 4-3" x Ø.131" nails, 4-3" 14 gage staples
LEDGER STRIP, FACE NAIL @ EA JOIST....3-16d Com, 4-3" x Ø.131" nails, 4-3" 14 gage staples

WOOD STRUCTURAL PANELS AND PARTICLEBOARD:[b]
SUBFLOOR, ROOF AND WALL SHEATHING (TO FRAMING):
1/2" AND LESS....6d[c,l], 2 3/8" x Ø.113" nail[n], 1 3/4" 16 gage[o]
19/32" TO 3/4"....8d[d] OR 6d[e], 2 3/8" x Ø.113" nail[p], 2" 16 gage[p]
7/8" TO 1"....8d[c]
1 1/8" TO 1 1/4"....10d[d] OR 8d[e]
SINGLE FLOOR (COMBINATION SUBFLOOR-UNDERLAYMENT TO FRAMING)
3/4" AND LESS....6d[e]
7/8" TO 1"....8d[e]
1 1/8" TO 1 1/4"....10d[d] OR 8d[e]
PANEL SIDING (TO FRAMING)
1/2" OR LESS....6d[f]
5/8"....8d[f]
FIBERBOARD SHEATHING:[g]
1/2"....NO. 11 GA. ROOFING NAIL[h]; 6d[d]; NO. 16 GA. STAPLE[i]
25/32"....NO. 11 GA. ROOFING NAIL[h]; 8d[d]; NO. 16 GA. STAPLE[i]
INTERIOR PANELING
1/4"....4d[j]
3/8"....6d[k]

NOTES:

a. COMMON OR BOX NAILS ARE PERMITTED TO BE USED UNLESS OTHERWISE NOTED.
b. NAILS SPACED @ 6" ON CENTER AT EDGES, 12" AT INTERMEDIATE SUPPORTS EXCEPT 6" AT SUPPORTS WHERE SPANS ARE 48" OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTION 2305. NAILS FOR WALL SHEATHING MAY BE COMMON, BOX OR CASING.
c. COMMON OR DEFORMED SHANK. (6d - 2" x Ø.113", 8d - 2 1/2" x Ø.131", 10d - 3" x Ø.148")
d. COMMON. (6d - 2" x Ø.113", 8d - 2 1/2" x Ø.131", 10d - 3" x Ø.148")
e. DEFORMED SHANK. (6d - 2" x Ø.113", 8d - 2 1/2" x Ø.131", 10d - 3" x Ø.148")
f. CORROSION-RESISTANT SIDING (6d - 1 7/8" x Ø.106", 8d - 2 3/8" x Ø.128") OR CASING (6d - 2" x Ø.099", 8d = 2 1/2" x Ø.113") NAILS.
g. FASTENERS SPACED 3" O.C. @ EXTERIOR EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS, WHEN USED AS STRUCTURAL SHEATHING. SPACING SHALL BE 6" O.C. ON THE EDGES ∮ 12" O.C. @ INT. SUPPORTS FOR NONSTRUCTURAL APPS.
h. CORROSION-RESISTANT ROOFING NAILS WITH 7/16" DIAMETER HEAD ∮ 1 1/2" LENGTH FOR 1/2" SHEATHING AND 1 3/4" LENGTH FOR 25/32" SHEATHING.
i. CORROSION-RESISTANT STAPLES WITH NOMINAL 7/16" CROWN OR 1" CROWN AND 1 1/4" LENGTH FOR 1/2" SHEATHING AND 1 1/2" LENGTH FOR 25/32" SHEATHING. PANEL SUPPORTS @ 16" (20" IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, U.O. MARKED)
j. CASING OR FINISH NAILS SPACED 6" ON PANEL EDGES, 12" AT INTERMEDIATE SUPPORTS.
k. PANEL SUPPORTS @ 24". CASING OR FINISH NAILS SPACED 6" ON PANEL EDGES, 12" AT INTERMEDIATE SUPPORTS.
l. FOR ROOF SHEATHING APPS, 8d NAILS ARE THE MIN. REQ'D FOR WOOD STRUCTURAL PANELS
m. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16"
n. FOR ROOF SHEATHING APPS, FASTENERS SPACED @ 4" O.C. AT EDGES, 8" O.C. AT INT. SUPPORTS
o. FASTENERS SPACED 4" O.C. AT EDGES, 8" O.C. AT INT. SUPPORTS FOR SUBFLOOR AND WALL SHEATHING AND 3" O.C. AT EDGES, 6" AT INT. SUPPORTS FOR ROOF SHEATHING
p. FASTENERS SPACED 4" O.C. AT EDGES, 8" O.C. AT INT. SUPPORTS.
q. ALL 10d NAILS SHALL BE 10d COMMON (3" x Ø.148" MIN), 16d NAILS SHALL BE 3 1/2" x Ø.135" MIN, 16d COMMON NAILS SHALL BE 3 1/2" x Ø.162" MIN ∮ 20d COMMON NAILS SHALL BE 4" x Ø.192" MIN.

NOTE:
FASTENERS - INCLUDING NUTS AND WASHERS - FOR PRESERVATIVE TREATED WOOD (IN ALL APPLICATIONS) AND FIRE-RETARDANT-TREATED-WOOD (IN EXTERIOR APPLICATIONS) SHALL BE HOT DIPPED ZINC COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE, OR COPPER. (CRC R317.3)



NOTES:
1. MAXIMUM DEPTH OF NOTCH @ ENDS OF MEMBER = d/4
2. MAXIMUM DEPTH OF NOTCH IN SPAN LENGTH = d/6
3. NOTCHING IN MIDDLE 1/3 OF SPAN NOT ALLOWED.

JOIST OR BEAM NOTCH (1)

SPECIAL INSPECTIONS:

IN ADDITION TO THE INSPECTIONS REQUIRED BY IBC SEC 109, THE OWNER OR ARCHITECT SHALL EMPLOY SPECIAL INSPECTORS, PROVIDE SPECIAL INSPECTIONS DURING CONSTRUCTION AS SUMMARIZED IN THE FOLLOWING TABLE. THE CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SPECIAL INSPECTOR TO PERFORM THESE SPECIAL INSPECTIONS.

INSPECTIONS:
ALL CONSTRUCTION IS SUBJECT TO INSPECTION BY THE BUILDING OFFICIAL IN ACCORDANCE WITH IBC SEC 109. THE CONTRACTOR SHALL COORDINATE ALL REQUIRED INSPECTIONS WITH THE BUILDING OFFICIAL.

EPOXY DOWELS
1. NEW FTG REBAR DOWEL ∮ EPOXY HOLDOWN ANCHOR TO EXISTING CONC FOOTING PER DETAIL 4/S3 ∮ 4/S4 USING SIMPSON SET-XP EPOXY PER ICC ESR#2508

1. THE SPECIAL INSPECTIONS LISTED ARE IN ADDITION TO THE CALLED INSPECTIONS REQUIRED BY SECTION 109 OF THE IBC, AS AMENDED. SPECIAL INSPECTION IS NOT A SUBSTITUTE FOR INSPECTION BY A CITY INSPECTOR.
2. CONTINUOUS INSPECTION IS ALWAYS REQUIRED DURING THE PERFORMANCE OF THE WORK UNLESS OTHERWISE SPECIFIED. WHEN WORK IN MORE THAN ONE CATEGORY OF WORK REQUIRING SPECIAL INSPECTION IS TO BE PERFORMED SIMULTANEOUSLY, OR THE GEOGRAPHIC LOCATION OF THE WORK IS SUCH THAT IT CANNOT BE CONTINUOUSLY INSPECTED AS DEFINED IN CBC SECTION 1702, IT IS THE AGENT'S RESPONSIBILITY TO EMPLOY A SUFFICIENT NUMBER OF INSPECTORS TO ASSURE THAT ALL WORK IS INSPECTED IN ACCORDANCE WITH THOSE PROVISIONS.
3. THE SPECIAL INSPECTORS MUST BE CERTIFIED BY THE CITY OF JURISDICTION TO PERFORM THE TYPE OF INSPECTION SPECIFIED.
   EXCEPTIONS:
   a. SOILS INSPECTION BY THE ENGINEER OF RECORD.
   b. SMOKE CONTROL SYSTEM, BY THE MECHANICAL ENGINEER OF RECORD.
   c. WHEN WAIVED BY THE BUILDING OFFICIAL.
4. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE SPECIAL INSPECTOR OR INSPECTION AGENCY AT LEAST ONE WORKING DAY PRIOR TO PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION.
5. WORK REQUIRING SPECIAL INSPECTION THAT IS INSTALLED OR COVERED WITHOUT THE APPROVAL OF THE CITY INSPECTOR IS SUBJECT TO REMOVAL OR EXPOSURE AT NO COST TO THIS JURISDICTION.
6. A CERTIFICATE OF SATISFACTORY COMPLETION OF WORK REQUIRING SPECIAL INSPECTION MUST BE SUBMITTED TO THE FIELD INSPECTION DIVISION.
7. AN APPLICATION FOR OFF-SITE FABRICATION MUST BE SUBMITTED TO THE FIELD INSPECTION DIVISION FOR APPROVAL PRIOR TO FABRICATION.
8. A CERTIFICATE OF COMPLIANCE FOR OFF-SITE FABRICATION MUST BE COMPLETED ∮ SUBMITTED TO THE FIELD INSP. DIVISION PRIOR TO ERECTION OF PRE-FAB'D COMPONENTS.
9. THE APPROVED FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE IN ACCORDANCE WITH CBC SECTION 1704.2.2. WHEN WELDING IS DONE IN A SHOP WHICH IS NOT AN APPROVED FABRICATOR, SPECIAL INSPECTION IS REQUIRED AND AN APPLICATION TO PERFORM OFF-SITE FABRICATION MUST BE SUBMITTED TO AND APPROVED BY THE FIELD INSPECTION DIVISION.

DESIGN CRITERIA: 2015 I.B.C. ∮ 2016 C.B.C.

DESIGN CRITERIA: 2015 IBC & 2016 CBC

1. GRAVITY LOAD:

a. (N) ROOF OVER (E) ROOF	DEAD LOAD	26	PSF
	LIVE LOAD	20	PSF
b. (N) ROOF	DEAD LOAD	18	PSF
	LIVE LOAD	20	PSF
c. (N) ENTRANCE TOWER ROOF	DEAD LOAD	14	PSF
	LIVE LOAD	20	PSF
d. (N) FLOOR	DEAD LOAD	16	PSF
	LIVE LOAD	40	PSF

2. WIND LOAD: a. BASIC WIND SPEED = 110 MPH; b. EXPOSURE = B

3. SEISMIC: 2015 IBC & 2016 CBC SEISMIC DESIGN PARAMETERS

PARAMETER	VALUE	REFERENCE
SITE COORDINATES	33.127°	LATITUDE
	-117.076°	LONGITUDE
SEIS. DESIGN CATEGORY	D	2013 CBC Table 1613.3.5
BUILDING CATEGORY	II	ASCE7-10 Table 1.5-1
SOIL SITE CLASS	D	ASCE7-10 Table 20-3-1
RESPNS SPEC. ACC., S_S	1.037 g	USGS
RESPNS SPEC. ACC., S_1	0.401 g	USGS
SITE COEFFICIENT, F_a	1.085	2013 CBC T-1613.3.3(1)
SITE COEFFICIENT, F_v	1.599	2013 CBC T-1613.3.3(2)
MAX. ETHQK ACC. S_{MS}	1.125 g	EQ. 11.4-1 & USGS
MAX. ETHQK ACC. S_{M1}	0.641 g	EQ. 11.4-2 & USGS
DESIGN SEISMIC, S_{DS}	0.75 g	EQ. 11.4-3 & USGS
DESIGN SEISMIC, S_{D1}	0.427 g	EQ. 11.4-4 & USGS
IMPORTANCE FACTOR, I_E	1.0	ASCE 7-10 T-1.5-2
RESPNS MODIF. COEFF., R	6.50	ASCE7-10 T-12.2-1
GOVERN'G BASE SHEAR, V	0.105 W	ASD w/ ρ =1.3

4. FOUNDATIONS: CLASS 5 SOILS PER CBC TABLE 1806.2

a. ALLOW SOIL BEARING PRESSURE	=	1500 PSF
CLASS 4 SOILS PER CBC TABLE 1806.2		
b. LATERAL BEARING PRESSURE	=	150 PSF
c. FRICTION COEFFICIENT	=	0.25
d. SHORT TERM INCREASE	=	1.33

STRUCTURAL OBSERVATION

STRUCTURAL OBSERVATION SHALL BE PROVIDED BY DUNN SAVOIE INC. IN ACCORDANCE WITH SECTION 1702 ∮ 1710 OF THE CALIFORNIA BUILDING CODE FOR THE FOLLOWING:
1. FINAL FRAMING ONCE ALL FRAMING MEMBERS, SHEAR WALLS, PLYWOOD ∮ MISC. CONNECTIONS HAVE BEEN INSTALLED.

STRUCTURAL OBSERVATION IS REQUIRED IN ADDITION TO ALL OTHER INSPECTIONS

STRUCTURAL OBSERVATION MEANS THE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM, FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS, AT SIGNIFICANT CONSTRUCTION STAGES AND AT COMPLETION OF THE STRUCTURAL SYSTEM. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY C.B.C. SECTION 1702 ∮ 1704 OR OTHER SECTIONS OF THIS CODE.

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO REQUEST STRUCTURAL OBSERVATION BY THE ENGINEER. THE ENGINEER SHALL BE GIVEN A MINIMUM OF 24 HOURS NOTICE BY THE CONTRACTOR PRIOR TO PERFORMING STRUCTURAL OBSERVATION.

A STATEMENT IN WRITING WILL BE PROVIDED TO THE BUILDING OFFICAL, STATING THAT THE SITE VISITS HAVE BEEN MADE AND WHETHER OR NOT ANY OBSERVED DEFICIENCIES HAVE BEEN CORRECTED TO CONFORM TO THE APPROVED PLANS AND SPECIFICATIONS.

EXPANSION ∮ EPOXY ANCHORS

1. ALL ANCHORS SHALL BE I.C.C. APPROVED.
2. ALL ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S APPROVED RESEARCH REPORT.
3. CONCRETE EXPANSION ANCHORS SHALL BE ONE OF THE FOLLOWING:
   (a) SIMPSON STRONG-BOLT WEDGE ANCHORS (ICC #1771)
   (b) SIMPSON STRONG-BOLT 2 WEDGE ANCHORS (PER ICC ESR-3037)
   (c) HILTI KWIK BOLT 3 (KB3) (ICC# ESR 2302)
   (d) HILTI KB-T2 ANCHOR (PER ICC ESR-1917).
   INSTALL PER MANUFACTURER'S RECOMMENDATIONS. SPECIAL INSPECTION REQUIRED.
4. CONCRETE EPOXY ANCHORS SHALL BE:
   (a) SIMPSON SET-XP EPOXY (ICC ESR#2508)
   (b) HILTI HIT-RE 500-SD EPOXY (ICC ESR#2322).
   INSTALL PER MANUFACTURER'S RECOMMENDATIONS. SPECIAL INSPECTION REQUIRED.
5. CMU EPOXY ANCHORS SHALL BE:
   SIMPSON SET EPOXY (ICC ESR#1772)
   INSTALL PER MANUFACTURER'S RECOMMENDATIONS. SPECIAL INSPECTION REQUIRED.

GENERAL NOTES

1. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS ∮ CONDITIONS AT THE JOB SITE, AND SHALL BE RESPONSIBLE FOR CONDITIONS OF ALL WORK ∮ MATERIALS, INCLUDING THOSE FURNISHED BY SUB-CONTRACTORS. ANY DISCREPANCIES AND/OR VARIATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT ∮ STRUCTURAL ENGINEER.
2. DETAILS SHOWN ON DRAWINGS APPLY AT ALL LIKE CONDITIONS.
3. ALL MATERIALS AND WORKMANSHIP SHALL BE PERFORMED IN ACCORDANCE WITH THE LOCAL STANDARDS AND THE APPLICABLE PROVISIONS OF THE 2016 EDITION OF THE C.B.C.
4. WHERE CONTINOUS INSPECTION IS REQUIRED BY THESE PLANS, A REGISTERED DEPUTY INSPECTOR, APPROVED BY AND RESPONSIBLE TO THE ARCHITECT AND BUILDING DEPARTMENT, SHALL BE EMPLOYED BY THE OWNER.
5. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, UNLESS OTHERWISE INDICATED, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKMEN, AND OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO BRACING, SHORING FOR CONSTRUCTION. EQUIPMENT SHORING FOR THE BUILDING, SHORING FOR THE EARTH BANKS, FORMS, SCAFFOLDING, PLANKING, SAFETY NETS, SUPPORT AND BRACING FOR CRANES AND GIN POLES, ETC.. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND HE OR SHE SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT OR THE ENGINEER SHALL NOT CONSTITUTE INSPECTION OF THE ABOVE ITEMS.
6. THESE DRAWINGS SHOW ONLY REPRESENTATIVE AND TYPICAL DETAILS TO ASSIST THE CONTRACTOR. THE DRAWINGS DO NOT ILLUSTRATE EVERY CONDITION. ALL ATTACHMENTS, CONNECTIONS, FASTENINGS, ETC. TO BE PROPERLY SECURED IN CONFORMANCE WITH BEST PRACTICE, AND THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING AND INSTALLING THEM.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SELECTING METHODS OF DEMOLITION SO THAT THE REMAINING STRUCTURE WILL NOT BE IMPAIRED OR DAMAGED.

WOOD/LUMBER

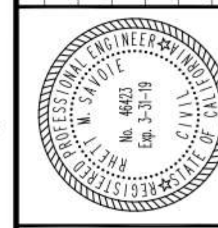
1. ALL WOOD MEMBERS SHALL BE DOUGLAS FIR/LARCH, CONFORMING TO THE CURRENT WESTERN WOOD PRODUCTS ASSOCIATION GRADE RULES BOOK, OR AS SPECIFIED OR AS CALLED FOR ON DRAWINGS. EACH PIECE OF LUMBER SHALL BE S4S SEASONED AND GRADE MARKED. ALL STUDS TO DOUGLAS FIR CONSTRUCTION GRADE OR BETTER.
2. UNLESS NOTED OTHERWISE ON THE DWGS., LUMBER GRADES SHALL BE AS FOLLOWS:
   A. VERTICAL FRAMING MEMBERS
      1. 2x STUDS, 3x STUDS ∮ 4x POSTS - NO. 2
      2. POSTS - 5" x 5" AND LARGER - NO. 1
      3. ALL OTHER VERTICAL MEMBERS - NO. 2
   B. HORIZONTAL FRAMING MEMBERS
      1. 2x ∮ 3x, U.O.N. - NO. 2
      2. ALL OTHER HORIZONTAL MEMBERS - NO. 1
3. ALL PLATES AND SILLS BEARING ON CONCRETE OR MASONRY SHALL BE PRESSURE TREATED DOUGLAS FIR, (P.T.D.F.) SEE SHEAR WALL SCHEDULE FOR ADDITIONAL NOTES. FASTENERS FOR PRESSURE TREATED WOOD SHALL BE OF HOT-DIPPED ZINC COATED GALVANIZED, STAINLESS STEEL, SILICON BRONZE OR COPPER
4. ALL STRUCTURAL SHEATHING (NOTED AS PLYWOOD, BUT OSB IS ACCEPTABLE) SHALL BE APA RATED SHEATHING EXP. 1 CONFORMING TO U.S. PRODUCT STANDARDS PS1-95, PS2-92 OR PRP 108, AND SHALL BE IDENTIFIED WITH APA GRADE MARK. SEE PLANS FOR THICKNESS AND PANEL IDENTIFICATION INDEX. STRUCTURAL SHEATHING AT EXPOSED DECKS AND WALKWAYS SHALL BE CDX PLYWD.
5. AT THE TIME OF INSTALLATION, ALL WOOD STRUCTURAL PANELS SHALL BE SPACED 1/8" AT PANEL ENDS AND EDGES.
6. 2" SOLID BLOCK SHALL BE PLACED BETWEEN ALL JOISTS AND RAFTER AT SUPPORTS.
7. PROVIDE 2X BLOCKING AT MIDHEIGHT OF STUDS OVER 10'-0" SEE SHEAR WALL SCHEDULE FOR ADDITIONAL REQUIREMENTS FOR SHEAR WALLS.
8. NAILS SHALL BE COMMON WIRE NAILS. UNLESS NOTED OTHERWISE ON THE PLANS. NAILING SHALL COMPLY WITH NAILING SCHEDULES PRESCRIBED BY TABLE 2304.9.1 OF THE BUILDING CODE.
9. LAG SCREWS: PREDRILL WITH A BIT SIZE OF 65% OF THE SHANK DIAMETER FOR THE THREADED PORTION. LEAD HOLES SHALL BE THE SAME LENGTH UNTHREADED PORTION AND THE SAME DIAMETER AS THE SHANK. SCREW ALL LAGS INTO PLACE. CUT WASHERS SHALL BE PROVIDED UNDER HEADS WHICH BEAR ON WOOD.
10. BOLTS SHALL COMPLY WITH ASTM A307. BOLT HOLES SHALL NOT BE MORE THAN 1/16" OVERSIZE. ALL BOLT HEADS AND NUTS BEARING ON WOOD SHALL HAVE STEEL WASHERS.
11. BOLTS IN WOOD SHALL NOT BE LESS THAN 7 DIAMETERS FROM THE END AND 4 DIAMETERS FROM THE EDGE UNLESS OTHERWISE DETAILED.
12. NO CHECKS OR SPLITS ALLOWED AT AREAS TO BE BOLTED.
13. ALL EXPOSED STEEL WOOD CONNECTORS, BOLTS, NUT ∮ WASHERS SHALL BE Z MAX OR HOT-DIPPED GALVANIZED.
14. WOOD CONNECTOR DESIGNATIONS SPECIFIED ON THE DRAWINGS REFER TO "SIMPSON STRONG-TIE CONNECTORS" AS MANUFACTURED BY THE SIMPSON CO. ∮ AS LISTED IN SIMPSON CATALOG NO. C-2011, SUBSTITUTIONS SHALL BE SUBJECT TO REVIEW AND APPROVAL BY THE ENGINEER PRIOR TO BID.
15. UNLESS OTHERWISE NOTED STRUCTURAL WALLS SHALL BE AS FOLLOWS:
    2x4 @ 16" O.C. U.O.N. ON PLAN (SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF 4' AND 6' WIDE NOMINAL WALLS.)
16. ROOF AND FLOOR SHEATHING SHALL BE LAID FACE GRAIN PERPENDICULAR TO SUPPORTS AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. WALL SHEATHING SHALL BE APPLIED WITH FACE GRAIN OR STRENGTH AXIS ORIENTED VERTICALLY.
17. GLUE AND NAIL ALL FLOOR SHEATHING TO SUPPORT FOR FIELD-GLUED FLOORS USE ADHESIVES MEETING APA SPECIFICATION AFG-01 OR ASTM D3498, APPLIED IN ACCORDANCE WITH THE MANUFACTURE'S RECOMMENDATIONS. IF O.S.B. PANELS WITH SEALED SURFACES OR EDGES ARE USED, USE ONLY SOLVENT-BASED GLUES, CHECK WITH PANEL MANUFACTURER.
18. ALL SAWN LUMBER FLOOR JOISTS SHALL BE S-DRY LUMBER (MOISTURE CONTENT NOT EXCEEDING 19%)
19. USE DOUBLE JOIST UNDER PARALLEL PARTITION WALLS
20. FASTENERS FOR PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOOD SHALL BE HOT DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE, OR COPPER. (CBC 2304.9.5)

ENGINEERED I- JOISTS (TJI'S) ∮ BEAMS (PL, ML ∮ TL)

1. ENGINEERED I-JOISTS AND BEAMS SHALL BE MANUFACTURED BY TRUS JOIST MacMILLAN, 1-800-628-3997. SUBSTITUTIONS SHALL BE SUBJECT TO REVIEW AND APPROVAL BY THE ENGINEER PRIOR TO BID.
2. I JOISTS ARE PER ICBO# PFC-4354 ∮ 5676P ∮ NER-119 ∮ 200
3. ON THE FRAMING PLANS, THE FOLLOWING NOMENCLATURE IS USED FOR THE ENGINEERED WOOD BEAMS: TL = TIMBERSTRAND LSL 1.5E, 1.3E OR FOR BLKG ∮ RIMS. ML = MICROLLAM LVL 1.9E. PL = PARALLAM PSL 2.0E
4. TJ - SHEAR PANELS ARE PER ICBO # ES PFC-5929

SHEET INDEX

Revisions:
△1 PLAN CHECK RESPONSE APR. 4, 2017



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Project Name:
RESENDIZ'S ROOM ADDITION & NEW ROOF STRUCTURE

Owner:

Designer:

MAR-Co.
HOUSE PLANS & DESIGNS
PLANS-DRAFTING-DESIGNS-3D-BUILDING PERMITS
MARCO A. LAUREANO
WWW.MARCOHOUSEPLANS.COM
760-805-2358
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Job No:	16-211
date drawn:	JAN.2017
date issued:	FEB.14,2017
project engineer:	AK
drawn by:	RJB
checked by:	RMS
scale:	AS NOTED

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(NOT FOR CONSTRUCTION)
☐ INFORMATION DWGS.
☐ PRELIMINARY DWGS.
☐ DESIGN DEVELOPMENT DWGS.
☐ CHECK SET
■ PROGRESS DRAWINGS
☐ PLAN CHECK DRAWINGS

☐ BID SET DRAWINGS
☐ CONSTRUCTION DOCUMENTS
☐ OTHER

sheet title:
GENERAL NOTES

sheet no.:
S-1